A Strategy for a Robust Design of Cracked Stiffened Panels

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Abstract—This work is focused on the numerical prediction of the fracture resistance of a flat stiffened panel made of the aluminium alloy 2024 T3 under a monotonic traction condition. The performed numerical simulations have been based on the micromechanical Gurson-Tvergaard (GT) model for ductile damage. The applicability of the GT model to this kind of structural problems has been studied and assessed by comparing numerical results, obtained by using the WARP 3D finite element code, with experimental data available in literature. In the sequel a home-made procedure is presented, which aims to increase the residual strength of a cracked stiffened aluminum panel and which is based on the stochastic design improvement (SDI) technique; a whole application example is then given to illustrate the said technique.

Keywords—Residual strength, R-Curve, Gurson model, SDI.

I. INTRODUCTION

The definition of prediction tools of the residual strength curve (R-curve) of cracked structures is a very interesting task to improve the chance to investigate on some basic fracture resistance aspects of the material behaviour. The approach considered within this work is a micromechanical one, based on continuum mechanics by considering the numerical model as independent from the geometry of a generic cracked component, which is able to describe the material behaviour from the initial damage conditions up to the final collapse. Ductile fracture begins in many metal alloys with the nucleation of cavities induced by the brittle breaking or decohesion of inclusions [1], [2]. As these cavities grow in size, they generate local intense stress-strain fields around near small inclusions, thereby nucleating small-scale cavities which participate to the final phase of the coalescence process and therefore to the macroscopic crack growth. The process of cavity growth is well understood and the relative models are quite advanced [3], [4], while the mechanism of nucleation and coalescence, as well as the associated micromechanics, are less well understood even if some papers provide a good description of such mechanisms [5], [6]. It is clear that improving the understanding of the above mechanisms and of their effects on failure modes and fracture resistance will result in a better ease to develop micromechanical prediction tools for the analysis of real components which behave in the nonlinear fracture mechanics field. In this work we have selected the micromechanical model introduced by Gurson [7] in the version modified by Tvergaard [8], whose parameters have been determined by using both experimental data from metallurgical observations provided from literature [9] and a phenomenological home-made fitting procedure which required combined experimental and numerical simulations.

The generally adopted structural optimization methods [10] aim to minimize one or more functions in presence of prescribed boundary conditions and with reference to the possible ranges where the some of the design variables are defined; at the same time the other structural parameters are considered as constant. Such a design process cannot take into account the scattering of those parameters which randomly influence both the manufacturing process and the service conditions and which, in turn, induce heavy effects on the variability of the performance of the design product [13]. That consideration clarifies the requirement of a special design methodology, based on probabilistic concepts, as well as complete with procedures and practical tools such as to make possible to explore in detail the probabilistic aspects involved in the design process of an industrial product [14] in such a way as to obtain a robust design whose behaviour is rather insensitive to all variations of the main variables, or, what is the same, a design whose statistics are characterized by the smallest standard deviation, as a function of the statistics of input [15].

In most recent years, in the field of the structural design the definition of robust design has been subject to a reanalysis, which has resulted in a new design technique called “stochastic design improvement” (SDI) [16]. The initial objective of the reduction of the standard deviation of the output has been replaced by the fulfillment of an assigned condition (target), defined by engineering or marketing considerations, to be reached within an assigned probability value.

In this work a home-made procedure has been developed, based on the SDI technique, which is able to perform a preliminary robust design of a complex structural component; this procedure is illustrated with reference to the case of a stiffened aeronautical panel [10], whose residual strength in presence of cracks has to be improved [17]. Numerical results on the reference component have been validated by using experimental results from literature [18].

II. DESCRIPTION OF THE HOME MADE SDI PROCEDURE

Perhaps the most used method to perform a probabilistic analysis of the behaviour of a complex structural component is Monte-Carlo (M-C) procedure, if necessary modified by using one among the variance reduction techniques in order to keep the number of necessary trials within an acceptable number.

On the other hand, in the case of optimization a M-C procedure based on a first trial assumption for the design variables gives a “cloud” of results, which is centred around a value which usually doesn’t coincide with the target; the SDI technique is based on the assumption that the same cloud can be displaced toward the desired position, i.e. in...
such a way as to be centred around the target, by varying only the mean value of the design variables and that the amplitude of the required displacement can be forecast through a close analysis of the points which are in the cloud, assuming that the shape and size of the cloud don’t change greatly if the displacement is small enough: it is therefore immediate to realize that an SDI process is composed by several sets of MC trials (runs) with intermediate estimates of the required displacement. It is also clear that the assumption about the invariance of the cloud can be maintained just in order to carry out the multivariate regression which is needed to perform a new step but that subsequently a new and more correct evaluation of the cloud is needed. The displacement of the cloud is obtained by changing the statistics of the design variables and in particular by changing their mean (nominal) values, as in the now available version of the method all distributions are assumed to be uniform, to avoid the gathering of results around the mode value. It is also pointed out that sometimes the process could fail to perform its task because of some physical (engineering) limit, but in any case SDI allows to quickly appreciate the feasibility of a specific design, therefore making its improvement easier.

It may also happen that other stochastic variables are present in the problem (the so called background variables): they can be characterized by any type of statistical distribution, but they are not modified during the process. From a practical standpoint, the designer specifies the value that an assigned output variable has to reach and the SDI process determines those values of the project variables which ensure that the objective variable becomes equal, in the mean sense, to the target. Therefore, the user defines, according to the requirements of the problem, a set of variables as control variables, which are then characterized from an uniform statistical distribution (natural variability) within which the procedure can let them vary, observing the corresponding physical (engineering) limits. In the case of a single output variable, the procedure evaluates the Euclidean or Mahalanobis distance of the objective variable from the target after each trial. Then, it is possible to find among the worked trials that one for which the said distance gets the smallest value: subsequently the procedure redefines each project variable according to a new uniform distribution with a mean value equal to that used in such “best” trial. The limits of natural variability are accordingly moved of the same quantity of the mean in such way as to save the amplitude of the physical variability. Once the project variables have been redefined a new run is performed and the process restarts up to the completion of the assigned number of shots. It is possible to plan a criterion of arrest in such way as to make the analysis stop when the distance from the target reaches a given value. In the most cases, it is desirable to control the state of the analysis with a real-time monitoring with the purpose to realize if a satisfactory condition has been obtained.

III. Test Case

The procedure described above has been applied on a flat cracked and stiffened panel (Fig. 1).

The full panel is constituted by a skin made of Al alloy 2024 T3 LT, divided in three bays by four stiffeners made of

<table>
<thead>
<tr>
<th>Material</th>
<th>E [MPa]</th>
<th>σy [MPa]</th>
<th>σu [MPa]</th>
<th>Δult [%]</th>
</tr>
</thead>
<tbody>
<tr>
<td>2024 T3 LT</td>
<td>71100</td>
<td>366</td>
<td>482</td>
<td>18</td>
</tr>
<tr>
<td>7075 T5 L</td>
<td>67000</td>
<td>525</td>
<td>579</td>
<td>16</td>
</tr>
</tbody>
</table>

Two different cracked panels have been considered with a central load through crack equal to 120 and 150 mm respectively. The full panels were tested by [18] by using a vertical hydraulic actuator with a maximum load capacity of 1000 kN. A double-bridge load cell was mounted at the rod of the actuator. The applied loads were controlled by a typical closed-loop servo system. The stiffened panel was clamped to the testing machine frames by 29 pins per side (20 mm diameter, Fig. 2); the stress field around pins does not reach the bearing strength of the stiffener material. Fig. 2 shows a scheme of panel mounted in the testing frame. Tensile rods were used to prevent horizontal deflection of the frame during loading with care given to the assembly process [24]. The residual strength tests were done under displacement control to make the crack statically grow beyond the point of maximum load. During the residual strength test, the displacement was gradually increased until failure of the panel.

After the experimental tests were completed, a finite element model has been developed and analysed by using the WARP 3D FE code. The numerical model consists of no. 8400 8-noded 3-dof solid elements and 11450 nodes, only an eighth of the whole structure has been analysed due to the three symmetry planes. Classical metal plasticity models have been adopted: Mises yield surfaces with associated plastic flow, which allow for isotropic yield. Isotropic hardening behaviour has been considered according the mechanical characteristics reported in Table I.

Boundary conditions and crack dimensions were the same of experimental test.

The surrounding zone of the crack has been modelled by means of the Gurson-Tvergaard isotropic constitutive model for progressively cavitating elastic-plastic solids. The progressive degradation of the material strength properties in the fracture process zone due to micro-void growth to coalescence is modelled through the computational cell
concept suitable for quasi-static ductile fracture processes. Such constitutive model needs a calibration phase reported in the following paragraph.

As it is well known, the Gurson-Tvergaard (GT) model [7], [8] is represented by the following expression (1)

\[
\phi(q, \sigma_0, f, \sigma_m) = \frac{\sigma^2}{\sigma_0^2} + 2q_f \cosh \left( \frac{3q_f \sigma_m}{2\sigma_0} \right) - 1 - q_1 f^2 = 0
\]  

where \( \phi \) governs the strain induced voids nucleation rate, \( q_f \) is the equivalent plastic strain, \( \sigma_0 \) is the yielding stress of the material, \( q_1, q_2 \) and \( q_3 \) are the Tvergaard correction factors. The void volume fraction rate, \( df \), consists of two terms, \( df_{\text{nucleation}} \) and \( df_{\text{growth}} \), respectively linked to the nucleation and the growth of voids. Void coalescence is assumed to start beyond a certain value of \( f \), say \( f_c \), and a macroscopic crack appears when the material ligaments between voids lose completely their capacity to carry a load whatever. From a numerical point of view, “computational cells”, which implement the GT model, are positioned adjacent to the crack propagation plane and are numerically characterized by means of the aforesaid parameters. The numerical characterization of the computational cell, obviously, determines the size of the finite elements used to model the area around the crack tip, by considering one finite element for each cell. Nine parameters need to be calibrated (by means of a fitting procedure of numerical results with experimental ones) in order to fully characterize the computational cell in the sense above: the three Tvergaard correction parameters \( q_1, q_2, q_3 \); the three parameters associated with the strain normal distribution (mean value, \( \varepsilon_{\text{m}} \), standard deviation, \( S_N \), and the volume fraction of void nucleating particles, \( k_i \)), which are assumed to govern the strain induced voids nucleation rate on the basis of the following expressions (2):

\[
df_{\text{nucleation}} = A(\bar{\varepsilon}) d\bar{\varepsilon}_p
\]

\[
A = \frac{f_0}{S_N\sqrt{2\pi}} \exp \left[ -\frac{1}{2} \left( \frac{\bar{\varepsilon}_p - \bar{\varepsilon}_N}{S_N} \right)^2 \right]
\]

where \( \bar{\varepsilon}_p \) is the equivalent plastic strain, \( D_0 \) is the initial size of the computational cell and \( f_0 \) is the initial volume cavity fraction which can reach its critical value \( f_c \). Another purely numerical parameter, \( \gamma \), has been considered, which governs the release model for element forces after the void volume fraction reaches the critical value [25]. In fact, at any load step after attaining the critical damage state, the remaining fraction of internal forces applied to nodes of the considered element at crack tip, \( \gamma \), is given by:

\[
\gamma = 1.0 - [D^* - D_0^*] / \lambda D_0
\]

where \( D_0^* \) is the average deformed cell height normal to the crack plane when \( f \) is equal to \( f_c \). \( D^* \) is the actual deformed cell height and \( \lambda D_0 \) represents the allowable elongation of the cell size from the critical condition up to the final cell collapse (\( \gamma = 0 \)), with respect to the undeformed cell height [25]. Beside these parameters it is obviously necessary to know the mechanical properties of the base material (Young modulus, \( E \), Poisson ratio, \( \nu \), yielding stress, \( \sigma_0 \), strain hardening, \( n \)), or its stress-strain relationship (\( \sigma - \varepsilon \) curve).

The investigated material has been the aluminium alloy 2024 T3 (Table I). On the base of this data, the fitting of Tvergaard’s parameters was performed by comparing two different numerical models under opportune boundary traction; the obtained values are \( q_1 = 1.33 \), \( q_2 = 0.956 \) and \( q_3 = 1.77 \), confirming the usual assumption \( q_3 = q_1^2 \). In that phase of the fitting process, the nucleation phenomena has not been considered and the \( f_c \) value has been used only to determine the last point of comparison between the stress-
strain curves of the two models without any influence on the fitting process. The second phase of the GT model parameters calibration process consists in the determination of the nucleation \((\varepsilon_N, S_N, f_N)\) and macro-mechanical \((D_0, f_0, f_c)\) parameters. Experimental data related to the considered material are needed to reach this aim, regarding both a metallographic analysis [9] and an R-curve of the material under examination [18], [26], [27]. For what concerns the metallographic analysis, the defect distribution in the base material is necessary to choose a first attempt value of the computational cell size, \(D_0\) and of the initial void volume fraction, \(f_0\), to be used in the calibration process. As it is possible to observe from the data recorded in Table II [9], particles or dispersoids are found, which, as it is known, are a cause for void nucleation and therefore can be considered as initial void volume fraction \((f_0 = 2.1\%)\). The average distance between the two biggest (> 10 \(\mu\)m) particles is 82.89 \(\mu\)m, which should be approximately the size of the computational cell.

With regard to this proposal it must be said that the smaller the computational cell size, the better is the agreement between the experimental and numerical results, but in order to approach the study of complex full scale structures a too small size of the computational cell may constitute a serious problem for what concerns the computational time; therefore, on the basis of the results reported in [28] and of numerical calculations performed by the authors in order to assess those results, a computational cell size \(D_0 = 100\ \mu\)m has been considered. In order to calibrate \(f_0\) and \(f_c\) parameters, numerical data have been fitted to the experimental ones represented by the R-curve of a flat stiffened panel of the Fig. 1. By considering just one crack in the middle bay of initial size \(a_0\) equal to 120 and 150 mm, the results reported in Figs. 4 and 5 have been carried out from both numerical and experimental analyses. As it is possible to observe, numerical results are in very good agreement with the experimental ones.

Table II: Results from metallographic analysis

<table>
<thead>
<tr>
<th>Equivalent diameter [(\mu\m)]</th>
<th>Volume fraction</th>
<th>Standard deviation</th>
<th>Nearest Neighbour [(\mu\m)]</th>
<th>Nearest Neighbour (st dv) [(\mu\m)]</th>
</tr>
</thead>
<tbody>
<tr>
<td>All Sizes</td>
<td>2.10</td>
<td>0.40</td>
<td>8.58</td>
<td>5.37</td>
</tr>
<tr>
<td>1:2</td>
<td>0.18</td>
<td>0.06</td>
<td>15.62</td>
<td>10.78</td>
</tr>
<tr>
<td>2:3</td>
<td>0.29</td>
<td>0.07</td>
<td>20.95</td>
<td>13.07</td>
</tr>
<tr>
<td>3:4</td>
<td>0.33</td>
<td>0.09</td>
<td>28.23</td>
<td>17.69</td>
</tr>
<tr>
<td>4:6</td>
<td>0.49</td>
<td>0.14</td>
<td>26.38</td>
<td>14.85</td>
</tr>
<tr>
<td>6:8</td>
<td>0.35</td>
<td>0.15</td>
<td>43.01</td>
<td>25.19</td>
</tr>
<tr>
<td>8:10</td>
<td>0.22</td>
<td>0.15</td>
<td>69.60</td>
<td>42.43</td>
</tr>
<tr>
<td>10+</td>
<td>0.24</td>
<td>0.24</td>
<td>82.89</td>
<td>54.35</td>
</tr>
</tbody>
</table>

V. STIFFENED PANEL: EXPERIMENTAL TEST AND NUMERICAL SIMULATIONS

The values obtained through the calibration process discussed in the previous section have been used to perform calculation of the R-curve of a flat stiffened panel of the Fig. 1. By considering just one crack in the middle bay of initial size \(a_0\) equal to 120 and 150 mm, the results reported in Figs. 4 and 5 have been carried out from both numerical and experimental analyses. As it is possible to observe, numerical results are in very good agreement with the experimental ones.

Fig. 3 Fitting parameters panel

Fig. 4 Stress vs. Half crack length

Fig. 5 Stress vs. Half crack length
height have been assumed, while the engineering target of variability have considered to be respectively [306 ± 374 mm] and [1.03 ± 3.09 mm]. An increment of the maximum value of the residual strength curve (Rmax) of the 12% with a success probability greater than 0.80 has been assumed as the target.

VI. ANALYSIS OF RESULTS AND CONCLUSION

A total of 6 runs, each one of 15 shots, have been considered adequate to satisfy the target, even if at the end of the procedure an extended MC must be performed in order to assess the obtained results from the 15 shots of the last satisfying run. In the following Figs. 6 and 7 the design variables vs. the maximum value of the residual strength has been illustrated. In correspondence to these two plots we recorded in Fig. 8 the values assumed by the maximum value of the R-curve for each shot. In the same figure the reference value (obtained by considering the initial nominal value of the design variables) of the maximum value of the R-curve is reported together with the target value (dashed line). As it is possible to observe, 9 of the 15 shots of the 5th run overcame the target value; it means that by using the corresponding mean value of the design variable the probability to satisfy the target is of about 0.60. Therefore, another run (the 6th) has been carried out and just one shot doesn’t overcome the target value, so that the approximate probability to satisfy the target is about 0.93. The mean value of the design variables in the 6th run is respectively 316.6 mm for the stringer pitch and 2.86 mm for the stringer height; the mean value of the output variable is 122000 N. An extended MC (75 trials) has been performed on the basis of the statistics of the 6th run and the results showed in the Fig. 9 have been obtained, in terms of the maximum value of the residual strength obtained for each trial; in Fig. 10 its mean value and standard deviation vs. the number of the trial have been recorded.

The new mean of the output variable is 121800 N with a standard deviation of 2500 N and the probability to satisfy the target is exactly 0.81. At the end, in the last Fig. 11, the six R-curves corresponding to the six best shots for each run are reported, together with the reference R-curve.
VII. CONCLUSION

The numerical Gurson-Tvergaard micromechanical damage model has been applied to predict the R-curve of a full scale flat stiffened panel made of A1 alloy 2024 T3 L.T, after calibrating its parameters by means of a phenomenological fitting procedure between numerical and experimental results. The obtained values of those parameters allowed the GT model, in the version implemented in the WARP 3D code, to predict the R-curve of the analysed component very well.

REFERENCES