Critical Points of Prefabricated Reinforced Concrete Wall Systems of Multi-storey Buildings

J. Witzany, T. Čejka and R. Zigler

Abstract—With respect to the dissipation of energy through plastic deformation of joints of prefabricated wall units, the paper points out the principal importance of efficient reinforcement of the prefabricated system at its joints. The method, quality and amount of reinforcement are essential for reaching the necessary degree of joint ductility. The paper presents partial results of experimental research of vertical joints of prefabricated units exposed to monotonously rising loading and repetitive shear force and formulates a conclusion that the limit state of the structure as a whole is preceded by the disintegration of joints, or that the structure tends to pass from linearly elastic behaviour to non-linearly elastic to plastic behaviour by exceeding the proportional elastic limit in joints. Experimental verification on a model of a 7-storey prefabricated structure revealed weak points in its load-bearing systems, mainly at places of critical points around openings situated in close proximity to vertical joints of mutually perpendicularly oriented walls.

Keywords—dissipative energy, dynamic and cycling load repetitive load, working diagrams of joints

I. CRITICAL POINTS AND DISSIPATIVE ENERGY OF PREFABRICATED REINFORCED CONCRETE WALL SYSTEMS

The reliability and structural safety of prefabricated reinforced concrete wall systems exposed to extraordinary effects (explosion, fire, acts of terrorism) and dynamic effects (technological and induced seismicity, natural seismicity) rely on the plastic deformation mechanism of mainly the joints of load-bearing prefabricated members with simultaneous dissipation of energy. At this phase of the prefabricated load-bearing reinforced concrete wall systems’ action, absorption of energy occurs, and the necessary condition at this phase of the bond’s action (plastic action phase) is that the respective structural bond must not be completely eliminated from the load-bearing system. This presumes that the plastic deformation mechanism must prevail at critical points of the bearing system during the dissipation of energy. In the case of prefabricated wall systems, it is the vertical joints of prefabricated wall units exposed mainly to shear forces and the floor slab stiffness binding individual vertical wall members into a spatial load-bearing system that play the essential role in this perspective. Concerning the dissipation of energy, while applying the plastic shearing mechanism at these critical points it is vital to avoid a substantial decrease in the so-called return force and avoid any potential instabilities. With respect to the dissipation of energy through plastic deformation of joints of prefabricated wall units, efficient reinforcement of the prefabricated system at its joints is of principal importance. The method, quality and amount of reinforcement are essential for reaching the necessary degree of joint ductility.

The deformational characteristics of joints of bearing units may negatively affect the structural safety under repetitive and cyclic loading. In this respect, the major agents are, in particular, the vertical joints of load-bearing wall units that make up the fundamental parts of the load-bearing prefabricated system – load-bearing prefabricated walls. The degradation of vertical joints and their impaired stiffness in the phase of the appearance and development of cracks may be the cause of a severe drop in the stiffness of the bearing system resulting in the loss of its ability to resist external loading effects.

In accordance with ENV 1992-1-3:1994 the respective reinforcement (ties) should prevent the propagation of local damage and successive failure of the structure under extreme loading, such as the impact of an explosion, creating alternative ways of load transfer in the case of local damage. The degree and method of minimum vertical wall reinforcement are regulated by EN 1992 – 1 – 1. To assess local ductility along vertical joints of prefabricated units, the relevant code criteria must be met (Art. 5.11.3.4. EC 8).

Prefabricated planar systems are characterized by a deformation and failure mechanism under which planar elements shift in joints disintegrated by cracks, i.e. at so-called contact interfaces. Due to the fact that the deformations of elements as compared to the deformation of their joints are small, they can be solved for standard, mainly design loads, under the condition of linear elasticity theory and planar stress state. We may, therefore, formulate an assumption that the limit state of the structure as a whole is preceded by joint disintegration, or that the structure tends to pass from linearly elastic behaviour to non-linearly elastic to plastic behaviour by exceeding the proportional elastic limit in joints (Fig. 1, [1]).
The experimentally determined values of relative shifts in relation to the joint width (i.e. 100 mm to 150 mm) around the proportional elasticity limit $\delta_{u}$ range between $2.10^{-2}$ mm and $1.10^{-1}$ mm. The permanent deformation $\delta_{t}$ after exceeding $\delta_{u}$ is 50 to 80 % of the total deformation $\delta_{u}$, while the limit deformation of the vertical joint $\delta_{m}$ at reaching the load limit $T_{m}$ is 0.6 to 2.5 mm. The total deformation in the domain of residual load-bearing capacity of the joint, i.e. after exceeding the ultimate bearing capacity $T_{u}$, may reach 10 to 25 mm. At this phase of action, the joint shows stiffness in the order of $10^{-2}$ and below as compared to the joint stiffness in the linearly elastic domain ($T \in (0, T_{u})$); also, having exceeded the load at the proportional elastic limit $T_{u}$, the joint stiffness falls to $10^{-1}$ - $10^{-2}$ of the initial joint stiffness value. This drop in stiffness affects the increments (decrements) of normal stresses in some units of the load-bearing system.

Apart from achieving more truthful design models of the bearing system, the introduction of physical dependences of joint behaviour also allows solving problems related to the assessment of a system with non-linearly elastic behaviour of joints of bearing elements exposed to the effects of emergency loading.

II. EXPERIMENTAL RESEARCH OF PLASTIC SHEARING MECHANISM UNDER LOADING WITH REPETITIVE SHEAR FORCE

The capacity of dissipative joints of prefabricated wall units, which represent critical chains (points) of the prefabricated load-bearing wall system in terms of its resistance to extraordinary effects and seismic loading, may be validated by means of inelastic cyclic testing where the so-called plastic shearing mechanism under loading with repetitive shear force is verified (EC 8).

The results of experimental tests (Fig. 2, [2]) manifested that the joints of concrete elements are extremely sensitive to the effects of variable repetitive and cyclic loading, the joint failure occurring due to cyclic loading amounting to 40 - 70 % of the joint limit load. Time-related non-linear changes in joint properties, in relation to the loading history and plastification, may be of principal importance for the structural safety and serviceability of the bearing system in time.
which mainly relies on the amount and type of reinforcement or prestress ($\mu_\sigma$, $\sigma_{KT}$, $T_o$, $T_{op}$, $\sigma$). The number of cycles $n_{op}$ of repetitive loading by the force $T_{op}$ depends on the magnitude of the force $T_{op}$. With the growing force $T_{op}$ the number of cycles $n_{op}$ leading to the joint failure decreases (Fig. 3a). The number of cycles of repetitive loading is also significantly affected by the magnitude of the force $T_o$ permanently acting in the joint. With the growing interval ($T_{op}$, $T_{op}$) the number of cycles of repetitive loading leading to the joint failure decreases. The results of experimental research proved that mainly in the domain of low reinforcement ($\mu_\sigma$, $\sigma_{KT} \in (0.1; 0.3)$ Nmm$^{-2}$) the joint prestress ($\sigma_L$) may affect the number of cycles $n_{op}$ more dramatically than reinforcement ($\mu_\sigma$, $\sigma_{KT} = - \sigma_L$). This may be explained by the fact that reinforcement in the joint starts to act only under deformations approaching the value of $\delta_{op}$. In this phase, the joint has already been damaged by diagonal cracks (Fig. 3b) and as a result of concrete disintegration in the vicinity of cracks (loosening of grains) shear stress is "poured" into the surroundings of the joint transverse reinforcement where – due to the "dowel-like" effect of reinforcement ("splitting" force around reinforcement) – the reinforcement successively tends to loosen.

After exceeding the joint’s proportional elastic limit (at least in one load cycle), repetitive – cyclic – effects cause gradual degradation of the joint, a gradual growth in deformations (strain) in joints leading up to a complete failure of joints (low-cycle fatigue, incremental collapse).

III. CONCLUSION

While assessing the resistance of prefabricated wall systems to extraordinary dynamic effects (caused e.g. by technical, induced and natural seismicity) potential degradation of joints due to cyclic deformations (loading) beyond the yield (proportionality) limit must be assessed which is caused by the system’s response to external dynamic effects. The failure mechanism – collapse of the critical bond – occurs by reaching the limit deformation under repetitive (cyclic) loading with a force approaching the yield limit, not under limit loading – incremental collapse. This issue was the subject of experimental research performed within the Research Plan “Reliability, optimization and durability of building materials and structures”, including research on a model of a 7-storey prefabricated load-bearing wall system assembled in 1:3 scale [3].

Repetitive loading of the experimental model was carried out by a pair of steel ties exerting an inclined force with horizontand vertical components acting at the upper free end of the model. The vertical component stabilized the experimental model of the structure against tilting (exerting compressive stresses in bed joints and substituting the effect of the vertical load, Fig. 4, [4]). Experimental research was performed on a model of a 7-storey prefabricated planar system loaded by the combination of step-by-step monotonously growing static loads and repetitive loads. In the total of three load states, 27 monotonously growing static load stages and 24 dynamic stages were performed. Static loading was performed by a step-by-step monotonously growing load increasing from 10 to 30 kN acting on each tie. Once completed, static loading was followed by dynamic loading in a range of ca 80,000 vibrations with an oscillation frequency of 15 Hz between individual static cycles.
dynamic effects caused by technical seismicity (e.g. effects of
traffic, Fig.5) [4]. Experimental verification revealed weak
points in load-bearing systems, mainly at places of critical
points around openings situated in close proximity to vertical
joints of mutually perpendicularly oriented walls.

IV. SUMMARY

The experimental and theoretical research completed to-
date has manifested a relatively high resistance of
prefabricated wall systems of multi-storey buildings with
discrete arrangements of reinforcement securing the joints
between individual load-bearing prefabricated units at the
floor slab level (Fig. 6) to the effects of common technical
seismicity with a frequency spectrum of seismic response and
a magnitude of seismic loading within the verified range.
Analogical results, however, cannot be expected for the case
of natural seismicity characterized, unlike technical seismicity,
by a relatively low frequency in the order of units and large
amplitudes.

Fig. 6 Scheme of join of “wall – ceiling structure – wall” in the
central part (a) and peripheral part (b) of the structure, c) scheme of
vertical joint of wall elements, d) joint of wall and floor unit, linking
bar, wall units faces coated with separation paint

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Jiří Witzany Professor, DrSc., Dr.h.c., Eng., Rector Emeritus, Department
of Building Structures, Faculty of Civil Engineering, Czech Technical
University in Prague, Czech republic. Professional activities during last 10 years: Chief researcher of 2 research plan and 8 grant, author and co-author of 10 monographs, 3 utility designs, 16 patents, 105 scientific and technical articles and over 60 papers on domestic and international conferences, 48 research reports. He devotes himself to structural problems of designing building structures, has carried out extensive theoretic and experimental research of prefabricated structures, reconstruction and rehabilitation designs of concrete and masonry buildings, degradation processes, durability and reliability of buildings. He has designed the reconstruction concept of Charles Bridge and 8 original prefabricated reinforcement concrete structures.

Tomáš Čejka Ph.D., Eng. Assistant lecturer at the Department of Building Constructions, Faculty of Civil Engineering, Czech Technical University in Prague, Czech republic. Professional activities during last 10 years: Co-researcher of 2 research plan and 8 grant, author and co-author of 8 monographs, 4 utility designs, 35 scientific and technical articles and over 45 papers on domestic and international conferences, 18 research reports. His research interests include structural analysis, mainly of masonry and precast concrete structures, reconstructions and renovations of buildings etc.

Radek Zíglar Ph.D., Eng. Eng. Assistant lecturer at the Department of Building Constructions, Faculty of Civil Engineering, Czech Technical University in Prague, Czech republic. Professional activities during last 10 years: Co-researcher of 2 research plan and 8 grant, co-author of 8 monographs, 5 utility designs, 35 scientific and technical articles and over 45 papers on domestic and international conferences, 18 research reports. His research interests include structural analysis, mainly of masonry and precast concrete structures, reconstructions and renovations of buildings etc.